DESIGN:

1. BUILDING CODE - 2018 INTERNATIONAL BUILDING CODE
2. ROOF DEAD, ROOF LIVE AND RAIN LOADS - REFER TO DRAWING S3.0
3. SNOW LOADS:
   - GROUND SNOW LOAD, Pg = 5 PSF
   - SNOW EXPOSURE FACTOR, Ce = 1.0
   - SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
   - THERMAL FACTOR, Ct = 1.0
4. WIND LOADS:
   - ULTIMATE WIND SPEED = 115 MPH
   - NOMINAL WIND SPEED = 90 MPH
   - RISK CATEGORY = II
   - OVERALL EXPOSURE CATEGORY = C
   - BUILDING CATEGORY = ENCLOSED
5. COMPONENTS AND CLADDING WIND DESIGN PRESSURES (GROSS): PER TABLE BELOW.

COMPONENTS AND CLADDING (ULTIMATE) WIND DESIGN PRESSURES (PSF)

	ZONE*	EFFECTIVE WIND AREA (SF)	WINDWARD PRESSURE	LEEWARD PRESSURE
ROOF	1	10	16.0	-30.7
		20	16.0	-29.9
		50	16.0	-28.9
		100	16.0	-28.1
	2	10	16.0	-51.5
		20	16.0	-46.1
		50	16.0	-38.8
		100	16.0	-33.3
	3	10	16.0	-77.5
		20	16.0	-64.2
		50	16.0	-46.6
		100	16.0	-33.3
WALLS	4	10	30.7	-33.3
		20	29.3	-31.9
		50	27.5	-30.1
		100	26.1	-28.6
	5	500	22.8	-25.5
		10	30.7	-41.2
		20	29.3	-38.3
		50	27.5	-34.7
		100	26.1	-31.9
		500	22.8	-25.5

6. SEISMIC LOADS:
   - RISK CATEGORY = II
   - SEISMIC IMPORTANCE FACTOR, Ie = 1.0
   - MAPPED SPECTRAL RESPONSE COEFFICIENTS:
     - Ss = 0.217
     - S1 = 0.084
   - SITE CLASS = D
   - SPECTRAL RESPONSE COEFFICIENTS:
     - Sds = 0.231
     - Sd1 = 0.134
   - SEISMIC DESIGN CATEGORY = C
   - BASIC SEISMIC-FORCE-RESISTING SYSTEM = INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
   - RESPONSE MODIFICATION COEFFICIENT, R = 3.5
   - ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE
   - SEISMIC RESPONSE COEFFICIENT, Cs = 0.047
   - BASE SHEAR, V = 37.3K

FOUNDATIONS:

1. THE FOUNDATION HAS BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS MADE IN THE "SOILS AND FOUNDATION INVESTIGATION" PREPARED BY _____
2. SPREAD FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF ____ PSF FOR INDIVIDUAL COLUMN FOOTINGS AND ____ PSF FOR CONTINUOUS WALL FOOTINGS UNDER FULL SERVICE LIVE AND DEAD LOAD.
3. FOOTINGS MAY BE POURED INTO AN EARTH-FORMED TRENCH IF SOIL CONDITIONS PERMIT.
4. ALL BEARING MATERIAL SHALL BE INSPECTED BY THE INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
5. BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF ____" BELOW FINAL EXTERIOR GRADE FOR FROST PROTECTION.
6. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.
7. WHERE FOUNDATION WALLS ARE TO HAVE EARTH PLACED ON EACH SIDE, PLACE FILL SIMULTANEOUSLY SO AS TO MAINTAIN A COMMON ELEVATION ON EACH SIDE OF THE WALL.
8. VERIFY USE AND EXTENT OF PERIMETER INSULATION WITH ARCHITECTURAL DRAWINGS PRIOR TO INSTALLATION OF FOUNDATIONS. INSTALL PERIMETER INSULATION AS REQUIRED.

CONCRETE:

1. CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH AND DENSITY, IN ACCORDANCE WITH THE FOLLOWING:

	STRENGTH (PSI)	DENSITY (PCF)
INTERIOR SLABS	4000	145
ALL CONCRETE EXPOSED TO WEATHER	4000	145
ALL OTHER CONCRETE (U.N.O.)	3000	145

2. SLUMP OF CONCRETE SHALL NOT EXCEED 4" UNLESS A HIGH RANGE WATER-REDUCING ADMIXTURE IS USED. THE SLUMP OF CONCRETE PRIOR TO ADDITION OF A HIGH RANGE WATER-REDUCING ADMIXTURE SHALL NOT EXCEED 4". THE SLUMP OF CONCRETE CONTAINING A HIGH RANGE WATER-REDUCING ADMIXTURE SHALL NOT EXCEED 10".
3. CONCRETE EXPOSED TO WEATHER, IN AREAS SUBJECT TO FROST, SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE BETWEEN 4 AND 8 PERCENT.
4. THE COARSE AGGREGATE SIZE SHALL BE # 57 OR LARGER. SEE CONCRETE SPECIFICATION FOR MAXIMUM AGGREGATE SIZE ALLOWED (3/4" MAXIMUM AT INTERIOR SLABS).
5. THE MINIMUM PORTLAND CEMENT (TYPE I OR II) CONTENT (ASTM C150) OF ALL CONCRETE SHALL CONFORM TO THE FOLLOWING TABLE: (FLY ASH NOT PERMITTED)

SPECIFIED COMPRESSIVE STRENGTH (PSI)	MINIMUM CEMENT CONTENT (POUNDS PER CUBIC YARD)	
	NON AIR-ENTRAINED CONCRETE	AIR-ENTRAINED CONCRETE
3000	470	517
4000	564	611

6. REINFORCING SHALL CONFORM TO ASTM A615, GR60 UNLESS NOTED OTHERWISE.
7. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
8. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED, IN ACCORDANCE WITH ACI DETAILING MANUAL 2004.
9. ALL REINFORCING SHALL BE SUPPORTED IN FORMS, SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER, IN ACCORDANCE WITH CRSI "MANUAL OF STANDARD PRACTICE" (2009).
10. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:
    - UNFORMED SURFACE IN CONTACT WITH THE GROUND: 3"
    - FORMED SURFACES EXPOSED TO EARTH OR WEATHER:
      - #6 BARS AND LARGER: 2"
      - #5 BARS AND SMALLER: 1 1/2"
    - FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:
      - BEAMS, GIRDERS, AND COLUMNS: 1 1/2"
      - SLABS, WALLS, AND JOISTS
      - #11 BARS AND SMALLER: 3/4"
      - #14 AND #18 BARS: 1 1/2"
11. LAP SPLICES SHALL BE IN ACCORDANCE WITH THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES:

BAR SIZE	TENSION SPLICES (INCHES)				COMPRESSION SPLICES (INCHES)
	TOP BARS		OTHER BARS		
	A	B	A	B	
#3	22	28	17	22	12
#4	29	37	22	29	15
#5	36	47	28	36	19
#6	43	56	33	43	23
#7	63	81	48	63	26
#8	72	93	55	72	30
#9	81	105	62	81	34
#10	91	116	70	91	38
#11	101	131	76	101	42

COMPRESSION DOWEL EMBEDMENT: 22 BAR DIAMETERS
LAP WELDED WIRE FABRIC ONE SPACING OF CROSS WIRES PLUS 2"

12. BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC. BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" OF CONCRETE.

REINFORCED MASONRY:

1. THE REINFORCED MASONRY FOR THIS PROJECT HAS BEEN DESIGNED AND DETAILED IN ACCORDANCE WITH THE STRENGTH DESIGN METHOD.
2. REINFORCED MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH, f'm, OF 1500 PSI. MASONRY UNITS SHALL BE NORMAL WEIGHT BLOCK CONFORMING TO ASTM C90, AND SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI. MORTAR SHALL CONFORM TO ASTM C270, TYPE S. GROUT SHALL CONFORM TO ASTM C476.
3. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
4. CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED TRUSS OR LADDER TYPE FORMED FROM 9 GAUGE COLD - DRAWN STEEL WIRE COMPLYING WITH ASTM A82.
5. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF VERTICAL CONTROL JOINTS. HORIZONTAL BOND BEAM AND LINTEL REINFORCING SHALL BE CONTINUOUS ACROSS VERTICAL CONTROL JOINTS. JOINT REINFORCING SHALL BE STOPPED EITHER SIDE OF VERTICAL CONTROL JOINTS.
6. ALL REINFORCED CELLS, ALL CELLS BELOW GRADE AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
7. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN 6 VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
8. VERTICAL BARS SHALL BE SECURED IN PLACE PRIOR TO GROUTING.
9. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM GROUT COVER OF 1/2 OF AN INCH TO THE INSIDE FACE OF MASONRY UNIT AND A MINIMUM TOTAL MASONRY COVER NOT LESS THAN TWO INCHES.
10. PARALLEL ADJACENT VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEAR DISTANCE NOT LESS THAN 1 1/2 BAR DIAMETERS NOR 1 1/2 INCHES.
11. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 3"x4".
12. GROUTING SHALL BE STOPPED 1-1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT.
13. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
14. ALL BOLTS, ANCHORS, ETC., INSERTED IN THE WALLS, SHALL BE GROUTED SOLID INTO POSITION.
15. REINFORCING BARS SHALL BE LAP SPLICED PER THE SCHEDULE ON SHEET S2.0.

STRUCTURAL STEEL:

1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING GRADES:

WIDE FLANGE SHAPES	A992 (Fy = 50)
CHANNELS, ANGLES, PLATES, ETC. (UNO)	A36 (Fy = 36)
STRUCTURAL TUBE	A500 (Fy = 46)
STEEL PIPE	A53 (Fy = 35)
ANCHOR RODS	F1554, A36 OR A307
BOLTS	A325
WELDING ELECTRODES	E70XX

2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE (2005), EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
3. THE STEEL STRUCTURE IS A NON-SELF-SUPPORTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE METAL ROOF DECK AND ATTACHMENT TO THE MASONRY WALLS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF PROVIDING THIS SUPPORT.
4. SPLICING OF STEEL MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
5. UNLESS NOTED OTHERWISE, BEAMS SHALL BEAR 8" MINIMUM ON CONCRETE OR MASONRY. UNLESS NOTED OTHERWISE, ANCHOR BEAMS TO MASONRY WITH TWO (2) 3/4" DIAMETER ANCHOR BOLTS WITH 1'-4" EMBEDMENT.

STEEL JOISTS: (REQUIRED VENDOR)

1. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE "STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, "K-SERIES" (2005) OF THE STEEL JOIST INSTITUTE (SJI).
2. STEEL JOISTS SHALL BE DESIGNED BY THE MANUFACTURER. THE MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN ADEQUACY AND SAFETY OF ALL STEEL JOISTS.
3. UNLESS OTHERWISE NOTED, STEEL JOISTS SHALL BE DESIGNED AS SIMPLY SUPPORTED. UNIFORMLY LOADED TRUSSES WITH THE TOP CHORD BRACED AGAINST LATERAL BUCKLING. THE UNIFORM DESIGN LOAD SHALL BE THE TOTAL SAFE UNIFORM LOAD AS SHOWN IN THE SJI STANDARD LOAD TABLE.
4. WHEN NET UPLIFT FORCES DUE TO WIND ARE SHOWN ON THE DRAWINGS, THE MANUFACTURER SHALL DESIGN THE JOISTS, BRIDGING AND CONNECTIONS TO THE SUPPORTING STRUCTURE FOR THE NET UPLIFT. A SINGLE LINE OF BOTTOM CHORD BRIDGING MUST BE PROVIDED NEAR THE FIRST BOTTOM CHORD PANEL POINTS WHENEVER NET UPLIFT DUE TO WIND FORCES IS SHOWN ON THE DESIGN DRAWINGS.
5. WHEN NONUNIFORM OR CONCENTRATED LOADS ARE SHOWN ON THE DRAWINGS, THE MANUFACTURER SHALL DESIGN THE JOISTS IN ACCORDANCE WITH PARAGRAPH 4.1 OF THE STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS, K-SERIES.
6. STEEL JOIST BRIDGING SHALL BE PROVIDED IN ACCORDANCE WITH THE SJI SPECIFICATION. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE PLACED AND STEEL JOIST ENDS FIXED PRIOR TO THE APPLICATION OF ANY LOADS. BRIDGING THAT TERMINATES AT, OR IS INTERRUPTED BY, STRUCTURAL STEEL BEAMS, MASONRY WALLS OR CONCRETE WALLS SHALL BE ATTACHED THERETO. COORDINATE BRIDGING LOCATIONS TO AVOID INTERFERENCE WITH ALL MECHANICAL, ELECTRICAL AND FIRE PROTECTION EQUIPMENT.
7. MINIMUM BEARING REQUIREMENTS, UNLESS NOTED OTHERWISE:
   - 2-1/2" ON STRUCTURAL STEEL
   - 4" ON STEEL BEARING PLATES OVER MASONRY OR CONCRETE
8. UNLESS NOTED OTHERWISE, STEEL JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL WORK OR STEEL BEARING PLATES WITH TWO (2) 1/8" FILLET WELDS (ONE EACH SIDE) 2" LONG, OR WITH A 1/2" DIAMETER BOLT.
9. STEEL JOISTS AT COLUMN CENTER LINES SHALL BE BOLTED TO STRUCTURAL STEEL WITH TWO (2) 1/2" DIAMETER BOLTS. WHERE STEEL JOISTS DO NOT SPACE TO COLUMN CENTER LINES, USE BOLTED CONNECTIONS FOR THE STEEL JOIST CLOSEST TO THE CENTER LINE.
10. HOLES IN STEEL JOIST CHORDS WILL NOT BE PERMITTED, EXCEPT FOR BOLTED CONNECTIONS AT THE BEARING END OF THE STEEL JOIST.
11. ALL ITEMS SUCH AS MECHANICAL EQUIPMENT, DUCT WORK, PIPES, CEILING, FIXTURES, ETC. THAT ARE TO BE SUPPORTED OR HUNG FROM THE STEEL JOIST SHALL BE FRAMED WITH AUXILIARY FRAMING TO THE PANEL POINTS OF THE STEEL JOIST. METHODS OF FRAMING THAT INDUCE BENDING TO THE STEEL JOIST CHORDS OR WEB MEMBERS WILL NOT BE PERMITTED.
12. REPAIR OR REPLACE ALL DAMAGED STEEL JOISTS.
13. STEEL JOISTS LOCATED NEAREST TO COLUMN CENTERLINES SHALL HAVE BOTTOM CHORD EXTENSIONS INSTALLED PER OSHA REQUIREMENTS.
14. ALL JOISTS IN BAYS 40 FEET OR LONGER SHALL HAVE THE ENDS CONNECTED TO THE SUPPORTING STEEL STRUCTURE WITH ERECTION BOLTS PER OSHA REQUIREMENTS.

STEEL JOIST GIRDERS: (REQUIRED VENDOR)

1. STEEL JOIST GIRDERS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR JOIST GIRDERS (2005) OF THE STEEL JOIST INSTITUTE (SJI).
2. STEEL JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER. THE MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN, ADEQUACY AND SAFETY OF ALL STEEL JOIST GIRDERS.
3. EXCEPT WHERE ADDITIONAL DESIGN LOADS ARE SHOWN ON THE STRUCTURAL DRAWINGS, STEEL JOIST GIRDERS SHALL BE DESIGNED AS SIMPLY SUPPORTED MEMBERS WITH ALL LOADS EQUAL IN MAGNITUDE AND EVENLY SPACED ALONG JOIST GIRDER TOP CHORD.
4. ADDITIONAL DESIGN LOADS FROM ROOF TOP EQUIPMENT OR OTHER CONCENTRATED LOADS SHOWN ON THE DRAWINGS SHALL BE CONSIDERED AS COLLATERAL LOADS. THESE LOADS SHALL BE CONSIDERED IN THE DESIGN OF THE STEEL JOIST GIRDERS. REFER TO THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND WEIGHT OF EQUIPMENT. WHERE SUCH LOADS DO NOT OCCUR AT THE PANEL POINTS OF THE STEEL JOIST GIRDER, AUXILIARY FRAMING SHALL BE ADDED OR THE TOP CHORD SHALL BE DESIGNED FOR THE EFFECTS OF THE LOAD.
5. STEEL JOIST GIRDERS SHALL BE DESIGNED TO SUPPORT THE DESIGN ROOF LIVE OR SNOW LOAD WITHOUT EXCEEDING A DEFLECTION OF L/240.
6. WHEN NET UPLIFT FORCES DUE TO WIND ARE SHOWN ON THE DRAWINGS, THE MANUFACTURER SHALL DESIGN THE STEEL JOIST GIRDERS AND CONNECTIONS TO THE SUPPORTING STRUCTURE FOR THE NET UPLIFT.
7. STEEL JOIST GIRDERS SHALL BE PROPORTIONED SUCH THAT THEY CAN BE ERECTED WITHOUT BRIDGING.
8. MINIMUM BEARING REQUIREMENT, UNLESS NOTED OTHERWISE:
   - 4" ON STRUCTURAL STEEL
   - 6" ON STEEL BEARING PLATES OVER MASONRY OR CONCRETE
9. UNLESS NOTED OTHERWISE, STEEL JOIST GIRDERS SHALL BE ATTACHED TO SUPPORTING STEEL WORK OR STEEL BEARING PLATES WITH TWO (2) 1/4" FILLET WELDS (ONE EACH SIDE), 2" LONG OR TWO (2) 3/4" DIAMETER BOLTS.
10. STEEL JOIST GIRDERS AT COLUMN CENTERLINES SHALL BE BOLTED TO STRUCTURAL STEEL WITH TWO (2) 3/4" DIAMETER BOLTS.
11. NO OTHER LOADS SHALL BE PLACED ON THE STEEL JOIST GIRDER UNTIL THE STEEL JOISTS BEARING ON THIS GIRDER ARE IN PLACE AND ATTACHED THERETO.
12. REPAIR OR REPLACE ALL DAMAGED STEEL JOIST GIRDERS.

METAL ROOF DECK: (REQUIRED VENDOR)

1. METAL ROOF DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE STANDARDS FOR STEEL ROOF DECK (2006).
2. METAL ROOF DECK SHALL BE CONFIGURATION, DEPTH AND MINIMUM GAGE AS SHOWN ON THE DRAWINGS. ATTACHMENT TO THE SUPPORTING STRUCTURE SHALL BE AS SHOWN ON THE DRAWINGS AS A MINIMUM. SEE ROOF FRAMING PLAN NOTES.
3. DO NOT HANG OR SUPPORT ANY LOADS FROM METAL ROOF DECK.
4. METAL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.

SPECIAL INSPECTIONS:

1. THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE THE REQUIRED SPECIAL INSPECTIONS DURING CONSTRUCTION.
2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
   A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.
   B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL-OF-RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, SUBMIT A COMPLETE LIST OF ALL OUTSTANDING DISCREPANCIES ON A WEEKLY BASIS TO THE OWNER, THE BUILDING OFFICIAL, AND THE PROFESSIONAL-OF-RECORD, UNTIL ALL CORRECTIONS HAVE BEEN COMPLETED.
   C. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE BUILDING CODE.
4. WHERE SPECIAL INSPECTION REQUIREMENTS DUPLICATE THE REQUIREMENTS OF OTHER SPECIFIED TESTING, DUPLICATE INSPECTIONS SHALL NOT BE REQUIRED.
5. STRUCTURAL OBSERVATION (AS DEFINED IN CHAPTER 17 OF THE BUILDING CODE) IS NOT REQUIRED, UNLESS SPECIFICALLY REQUIRED BY THE BUILDING OFFICIAL.
6. SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH THE FOLLOWING TABLES:

SOILS (IBC 1704.7)

VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	GEOTECHNICAL ENGINEERING REPORT
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC	
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC	
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS	
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC	

CONCRETE CONSTRUCTION (IBC1704.4)

VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD
1. INSPECTION OF REINFORCING STEEL AND PLACEMENT	PERIODIC	IBC1913.4; ACI 318: 3.5, 7.1-7.7
2. INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.	CONTINUOUS	IBC1911.5, 1912.1; ACI 318: 8.1.3, 21.2.8
3. INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.	PERIODIC	IBC1912.1; ACI 318: 3.8.6, 8.1.3, 21.2.8
4. VERIFYING USE OF REQUIRED DESIGN MIX	PERIODIC	IBC:1904.2.2, 1913.2, 1913.3; ACI 318: Ch. 4, 5.2-5.4
5. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	CONTINUOUS	IBC1913.10; ASTM C172; ASTM C31; ACI 318: 5.6, 5.8
6. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	CONTINUOUS	IBC:1913.6, 1913.7, 1913.8; ACI 318: 5.9, 5.10
7. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	IBC1913.9; ACI 318: 5.11-5.13
8. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 6.1.1

MASONRY CONSTRUCTION (TMS 402 LEVEL B)

VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD	
		TMS 402/ACI 530/ASCE 5	TMS 602/ACI 530.1/ASCE 6
1. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	PERIODIC		ART. 1.5
2. VERIFICATION OF f'm PRIOR TO CONSTRUCTION	PERIODIC		ART. 1.4B
3. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:			
a. PROPORTIONS OF SITE-PREPARED MORTAR.	PERIODIC		ART. 2.6A
b. CONSTRUCTION OF MORTAR JOINTS.	PERIODIC		ART. 3.3B
c. LOCATION OF REINFORCEMENT AND CONNECTORS.	PERIODIC		ART. 3.4, 3.6A
4. DURING CONSTRUCTION THE INSPECTION PROGRAM SHALL VERIFY:			
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.	PERIODIC		ART. 3.3F
b. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	PERIODIC	SEC. 1.2.2(e), 1.16.1	
c. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT AND ANCHOR BOLTS.	PERIODIC	SEC. 1.15	ART. 2.4, 3.4
d. WELDING OF REINFORCING BARS.	CONTINUOUS	SEC. 2.1.9.7.2, 3.3.3.4(b)	
e. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F.) OR HOT WEATHER (TEMPERATURE ABOVE 90°F.).	PERIODIC	IBC: 2104.3, 2104.4	ART. 1.8C, 1.8D
5. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:			
a. GROUT SPACE IS CLEAN.	PERIODIC		ART. 3.2D
b. PLACEMENT OF REINFORCEMENT AND CONNECTORS.	PERIODIC	SEC. 1.13	ART. 3.4
c. PROPORTIONS OF SITE-PREPARED GROUT.	PERIODIC		ART. 2.6B
d. CONSTRUCTION OF MORTAR JOINTS.	PERIODIC		ART. 3.3B
6. GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE.	CONTINUOUS		ART. 3.5

STEEL CONSTRUCTION (IBC 1704.3)

VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS:		
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	AISC 360, SECTION A3.3 AND APPLICABLE ASTM MATERIAL STANDARDS
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	PERIODIC	
2. INSPECTION OF HIGH-STRENGTH BOLTING:		
a. SNUG-TIGHT JOINTS.	PERIODIC	IBC1704.3.3; AISC 360, SECTION M2.5
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK :		
a. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	PERIODIC	AISC 360, SECTION M5.5
b. FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	PERIODIC	APPLICABLE ASTM MATERIAL STANDARDS
C. MANUFACTURER'S CERTIFIED TEST REPORTS.	PERIODIC	
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:		
a. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	AISC 360, SECTION A3.5 AND APPLICABLE AWS AS DOCUMENTS
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	PERIODIC	
TABLE CONTINUED ...		

STEEL CONSTRUCTION (IBC 1704.3)

TABLE CONTINUED ...

VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD
5. INSPECTION OF WELDING:		
a. STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:		
1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS	CONTINUOUS	IBC1704.3.1; AWS D1.1
2) MULTI-PASS FILLET WELDS	CONTINUOUS	
3) SINGLE-PASS FILLET WELDS > 5/16"	CONTINUOUS	
4) PLUG AND SLOT WELDS.	CONTINUOUS	
5) SINGLE-PASS FILLET WELDS ≤ 5/16"	PERIODIC	
6) FLOOR AND ROOF DECK WELDS	PERIODIC	AWS D1.3
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE:		
a. DETAILS SUCH AS BRACING AND STIFFENING	PERIODIC	IBC1704.3.2
b. MEMBER LOCATIONS		
c. APPLICATION OF JOINT DETAILS AT EACH CONNECTION		

INSPECTION OF FABRICATOR(S) (IBC 1704.2)

1. APPLICABLE ELEMENT (FABRICATOR CERTIFICATION REQUIREMENTS):
   a. STRUCTURAL STEEL (AISC CERTIFIED FOR CONVENTIONAL STEEL BUILDING)
   b. STEEL JOISTS/JOIST GIRDERS (SJI MEMBER)
   c. STEEL ROOF DECK (SDI MEMBER)
2. WHEN SPECIAL INSPECTIONS ARE REQUIRED BY BUILDING OFFICIAL:
   FABRICATION AND IMPLEMENTATION PROCEDURES: THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION, CONTROL OF THE WORKMANSHIP, AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL